


**Final Assessment Test – April 2026**

Course: BCLE305L - Transportation Engineering

Class NBR(s): 1302

Time: Three Hours

Slot: B1+TB1

Max. Marks: 100

- KEEPING MOBILE PHONE/ANY ELECTRONIC GADGETS, EVEN IN 'OFF' POSITION IS TREATED AS EXAM MALPRACTICE
- DON'T WRITE ANYTHING ON THE QUESTION PAPER

**General Instruction : Necessary formulae and guidelines from IRC are provided in Appendix**

| COs | CO Statements                                                                                     |
|-----|---------------------------------------------------------------------------------------------------|
| CO1 | Identify the most suitable highway alignment through engineering surveys.                         |
| CO2 | Design the geometric elements and understand various traffic elements on a highway.               |
| CO3 | Study the traffic conditions on a road and identify basic solutions for traffic problems.         |
| CO4 | Perform tests and identify whether a given highway pavement material is suitable for construction |
| CO5 | Design a highway pavement using IRC method.                                                       |

**BL – Blooms Taxonomy Level (1 – Remember, 2 – Understand, 3 – Apply, 4 – Analyse, 5 – Evaluate, 6 – Create)**
**PART – A (8 X 5 = 40 Marks)**  
**Answer ALL Questions**
**CO1 BL2**

1. In a city such as Vellore, some roads carry large volumes of traffic at higher speeds, while others mainly provide access to homes and local streets. Describe the functional classification of urban roads and write the role of each category.

**CO1 BL2**

2. An existing road between two towns passes through several villages and experiences congestion and delays. A proposal has been made to construct an alternative highway between the towns. There are two alignment options. Briefly discuss the procedure followed to evaluate whether the new highway is economically feasible and which alignment to be chosen. Note: Limit your answer to about 60 words.

**CO2 BL2**

3. Water stagnation on highway leads to pavement damage. Briefly describe the three design features that collectively provide good drainage of water so that water does not damage highways. Use appropriate schematic diagram(s).

**CO3 BL3**

4. Traffic volume count at a road section in peak hour resulted in the following data. Passenger car equivalency of each vehicle class are given. Calculate total volume in PCUs by 15-min interval and calculate the peak hour factor.

Table 1 Traffic volume count data

|                |               | Passenger Car Equivalency |               |           |       |
|----------------|---------------|---------------------------|---------------|-----------|-------|
|                |               | 0.4                       | 1.2           | 1         | 5     |
| Time period    | Vehicle class | Motorized Two-wheelers    | Autorickshaws | Cars/vans | Buses |
| 9am-9:15am     |               | 150                       | 57            | 171       | 85    |
| 9:15am-9:30am  |               | 210                       | 61            | 234       | 105   |
| 9:30am-9:45am  |               | 182                       | 62            | 190       | 90    |
| 9:45am-10:00am |               | 136                       | 58            | 182       | 110   |

5. Briefly describe the special features to be provided at the following locations to support movement of persons with disabilities.

CO2 BL2

- Parking areas
- Walking areas and crossings.

6. Compare impact value test and abrasion test which test the suitability of aggregates for highway construction. Follow the format given below:

CO4 BL3

| Aspect                            | Impact value test | Abrasion test |
|-----------------------------------|-------------------|---------------|
| Property of aggregate tested      |                   |               |
| Nature of loading being simulated |                   |               |
| Output value                      |                   |               |
| Passing criteria                  |                   |               |

7. Draw the cross-section of a typical rigid pavement and write what each layer contains. Write the function of each layer.

CO5 BL1

8. Large amounts of mobility data are now available from mobile phones, GPS devices, and CCTV cameras.

CO3 BL2

Describe any four transportation engineering applications of such data and briefly explain how the data would be used in each application.

Format:

| Application | How the data is used |
|-------------|----------------------|
| ....        | ....                 |

**PART – B (6 X 10 = 60 Marks)**

**Answer ALL Questions**

9. Calculate intermediate sight distance for a highway with design speed 70 km/h.

CO2 BL3

On this road, a raising section of 1 in 40 slope meets a falling section of 1 in 50 slope. Determine the vertical curve length at the joining of these sections such that at least intermediate sight distance is available for overtaking. Show your calculations.

- 10.a) A two-lane MDR is being designed for 65 km/h speed.

CO2 BL3

At a turning point shown in Figure 1, centrelline is at  $E=22$  m distance from Point of Intersection (O).  $\Delta=52$  deg.

Calculate the minimum horizontal curve radius allowed for safe turning at this speed. Calculate the radius of curve and design superelevation for this curve. Find maximum safe speed on this curve.

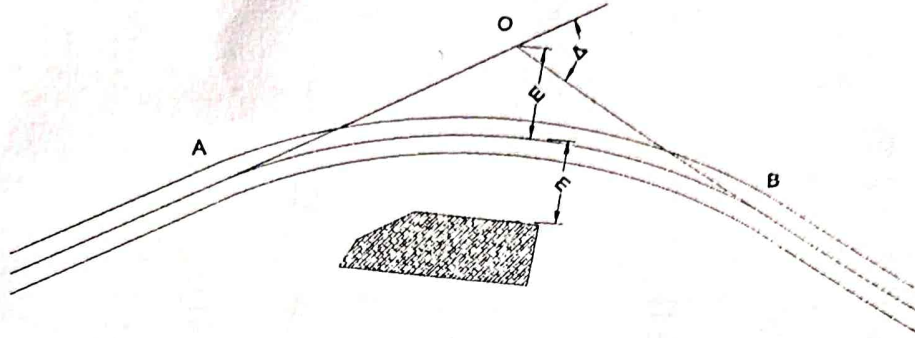


Figure 1 Horizontal curve

OR

- 10.b) Write the significance of set-back distance at a horizontal curve on a two-lane state highway as shown in Figure 1. For providing safe stopping sight distance ( $S$ ) on this curve with radius  $R$ , derive an expression for the minimum value of set-back distance,  $m$ , when length of curve  $L > S$ .

CO2 BL3

Assuming  $SSD=110$  m,  $\Delta=60$  deg., and radius= $120$  m, calculate the set-back distance required.

11. A two-lane highway currently (in 2026) carries an Annual Average Daily Traffic (AADT) of 13,900 vehicles. This contains 35% cars, 15% Motorcycles ( $PCE=0.3$ ), 15% autorickshaws ( $PCE=1.2$ ), 20% heavy vehicles ( $PCE=5$ ) and 15% buses ( $PCE=4.5$ ).

CO2 BL3

The highway is planned for capacity addition to meet traffic demand in the design year 2040. The traffic is expected to grow at a compound annual growth rate (CAGR) of 4.5%.

For design purposes, use peak hourly share  $K=0.095$  and Directional distribution factor  $D=0.55$ . The capacity of a single lane is estimated as 1950 PCU/hour.

Determine the number of lanes to be provided in the design year 2040 to ensure at least LOS C. Clearly justify your answer with calculations of Directional Design Hour Volume (DDHV) and Maximum Service Volume.

V/C ratio thresholds for different LOS from IndoHCM are: LOS A: 0.2, LOS B: 0.30, LOS C: 0.50, LOS D: 0.70 and LOS E: 1.0.

12. A highway (4-lane divided) connects with a town through a 2-lane road as shown in Figure 2. The highway authority found that conflicts were happening as vehicles from/to the town road interact with high-speed highway traffic. Draw and explain the necessary improvements needed at this intersection to mitigate this problem.

CO3

Note: Show and explain Physical improvements (ex: channelization);

Show and explain Signs and pavement markings

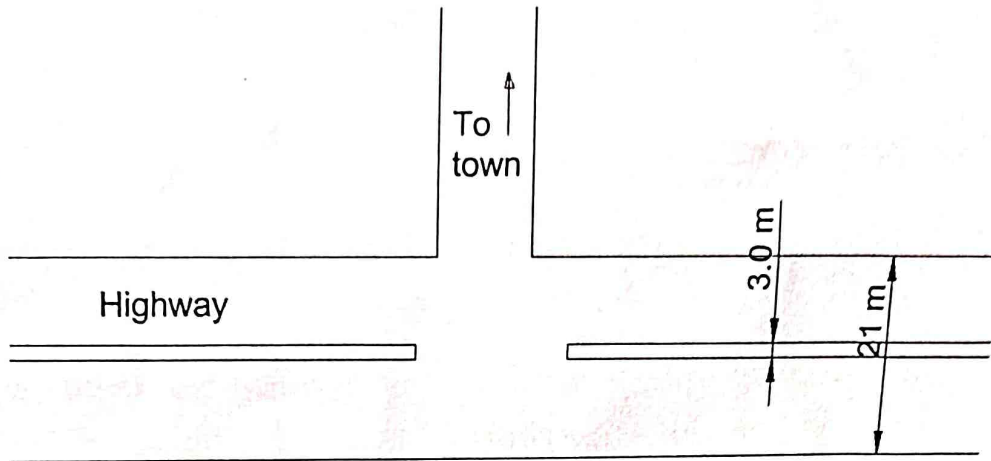


Figure 2 T- intersection at a highway connecting with a road to town

13. Bitumen is to be used for construction of a flexible pavement in a region experiencing high daytime temperatures and moderate rainfall.

CO4 BL3

- a) List four important properties of bitumen for this condition. For each, [6]  
write:

- The property's relevance to the given climate
- Name of the test to verify the suitability of bitumen for highway pavement
- What this test measures

- b) Two bitumen samples gave the following results: [4]

| Property            | Sample 1 | Sample 2 |
|---------------------|----------|----------|
| Penetration (0.1mm) | 80       | 40       |
| Softening point     | Low      | High     |

Which sample is more suitable for the situation given above? Justify based on expected pavement performance.

14.a) Answer the following;

CO5 BL3

- (i) Calculate the cumulative number of standard-axle-loads that would be applied on a two-lane highway pavement's design lane in a total design period of 15 years. [6]

AADT in the year of opening: 8000 veh/day

Annual traffic growth: 5% CAGR

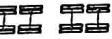
%share of commercial vehicles: 2-axle 4 tyre (Type A): 6%, 2-axle 6-tyre (Type B): 5%;

Vehicle damage factor (VDF): Type A: 1.90, Type B: 2.65.

- (ii) Using the following pavement performance equations from IRC 37-2018 equations, describe how the number of cumulative standard-axle-loads is used conceptually in design of flexible pavements. [4]

$$N_R = 4.1656 \times 10^{-8} [1/\epsilon^v]^{4.5337}$$
$$N_f = 1.6064 \times C \times 10^{-4} [1/\epsilon_t]^{3.89} [1/M_{Rm}]^{0.854}$$

OR

- 14.b) Describe the role of vehicle damage factor (VDF) in design of highway pavement. An axle load survey measured weights of both tandem axles of the five axle trucks (  )

CO5 BL3

Estimate the VDF for 5-axle trucks using the following information.

The survey obtained the axle load spectrum for tandem axles as shown in Table 2. Estimate the "total number of standard axles" and "axle equivalency factor" for tandem axle. If axle equivalency factor for front axles is 0.6, estimate the vehicle damage factor for 5-axle trucks.

Table 2

| Weight in tonnes | No. of TandemAxles |
|------------------|--------------------|
| 0 to <3          | 1                  |
| 3 to <6          | 3                  |
| 6 to <9          | 9                  |
| 9 to <12         | 19                 |
| 12 to <15        | 25                 |
| 15 to <18        | 23                 |
| 18 to <21        | 18                 |
| 21 to <24        | 8                  |
| 24 to <27        | 3                  |
| 27 to <30        | 1                  |

## Appendix

### Formulae:

#### Geometric design:

Circular curve formulae:  $\Delta$  = deflection angle;  $E$ =external distance= $R(\sec(\Delta/2)-1)$ ,

$M$ =middle ordinate= $R(1-\cos\Delta/2)$ ;

Length of curve:  $L=R\Delta\pi/180$ ;  $T=R \tan (\Delta/2)$

Horizontal curve at equilibrium:  $e+f_s = v^2/gR$ ,

$SSD = vt_R + v^2/2g(f \pm G)$ ,

Transition curve:

$L_s = 0.0215V^3/CR$ ,  $C=80/(75+V)$

$L_s = (W+W_e)/2 * e * N$

$F = P(1+r)^n$   $P = [(1+r)^n - 1]/[r(1+r)^n]$

Vertical curve length:

Summit curve:  $L > S$ :  $L = \frac{NS^2}{(\sqrt{2H} + \sqrt{2h})^2}$ , if  $L < S$ :  $L = 2S - \frac{(\sqrt{2H} + \sqrt{2h})^2}{N}$

Valley curve:  $L > S$ :  $L = \frac{NS^2}{(2h_1 + 2S \tan \alpha)}$ , if  $L < S$ :  $L = 2S - \frac{(2h_1 + 2S \tan \alpha)}{N}$

Highest or lowest point on curve:  $x = [n_1/(n_1+n_2)] * L$

Ordinates from tangent line along vertical curve ( $y$ ) at  $x$  is  $y = N/2L * x^2$

#### Traffic Engineering:

Greenshield's model:  $v = v_f - v_f(k/k_j)$  or  $k = k_j - k_j(v/v_f)$

#### Pavement Engineering:

Cumulative number of standard axles:  $N_{Des} = \frac{365 \times [(1+r)^n - 1]}{r} \times A \times D \times F$

Equivalent repetitions of standard axle loads = (Axle load in kN/standard axle load in kN)<sup>4</sup>

## Tables from IRC 73-2023

Table 3.2 Design speeds for NH/SH/MDR/ODR/VR

| S.No. | Nature of Terrain | Percent cross slope of the country | Design speed (km/h) (4/6/8-lane NH/SH) |         | Design speed (2-lane NH/SH) |         | Design speed (km/h) (MDR) |         | Design speed (km/h) (ODR/VR) |         |
|-------|-------------------|------------------------------------|----------------------------------------|---------|-----------------------------|---------|---------------------------|---------|------------------------------|---------|
|       |                   |                                    | Ruling                                 | Minimum | Ruling                      | Minimum | Ruling                    | Minimum | Ruling                       | Minimum |
| 1     | Plain             | 0-10                               | 100                                    | 80      | 100                         | 80      | 80                        | 65      | 50                           | 40      |
| 2     | Rolling           | 10-25                              | 100                                    | 80      | 100                         | 80      | 65                        | 50      | 50                           | 40      |
| 3     | Mountainous       | 25-60                              | 60                                     | 40      | 50                          | 40      | 40                        | 30      | 30                           | 20      |
| 4     | Steep             | >60                                | 60                                     | 40      | 40                          | 30      | 30                        | 20      | 30                           | 20      |

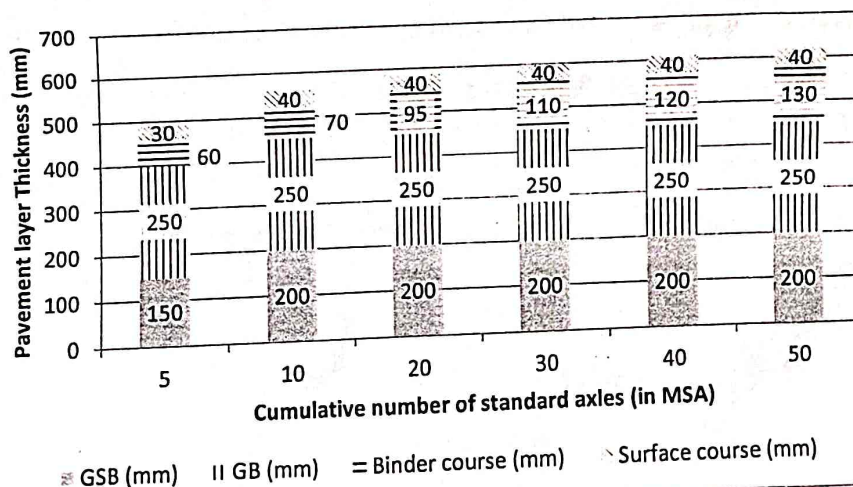
Table 7.5 Minimum Length of the Vertical Curve

| Design Speed (km/h) | Maximum Grade Change (percent) up to which Vertical Curve is not required | Minimum Length of a Vertical Curve (metre) |
|---------------------|---------------------------------------------------------------------------|--------------------------------------------|
| Up to 35            | 1.5                                                                       | 15                                         |
| 40                  | 1.2                                                                       | 20                                         |
| 50                  | 1.0                                                                       | 30                                         |
| 60                  | 0.8                                                                       | 40                                         |
| 80                  | 0.6                                                                       | 50 / 70*                                   |
| 100                 | 0.5                                                                       | 60 / 85*                                   |
| 120                 | 0.5                                                                       | 100*                                       |

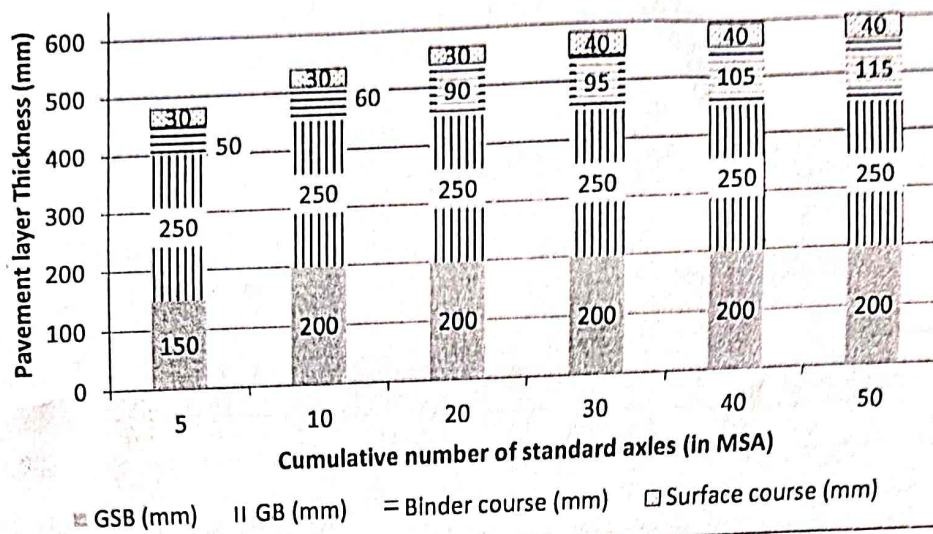
Note: Minimum length of vertical curves shown in asterisk (\*) are applicable for expressways.

## IRC 37-2018 Flexible Pavement Design Catalogues

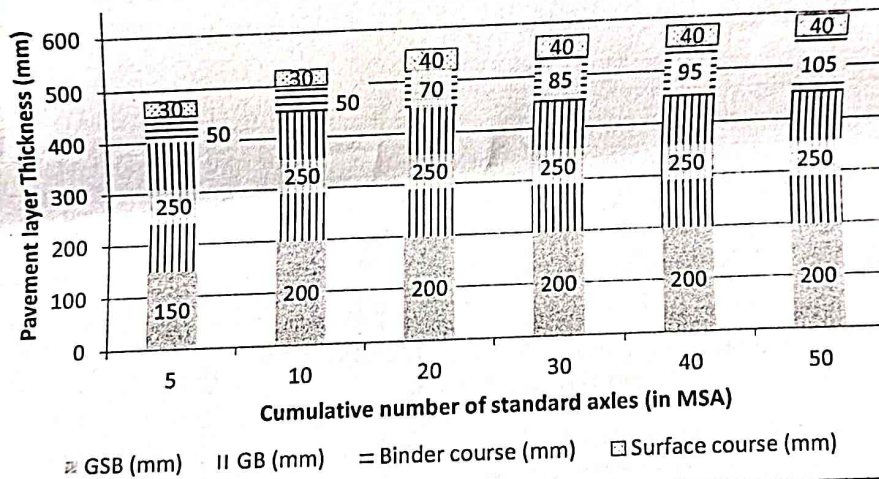
### Catalogue for pavement layer thicknesses - Effective CBR 6%



### Catalogue for pavement layer thicknesses - Effective CBR 8%



### Catalogue for pavement layer thicknesses - Effective CBR 10%



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